Streamgauging Planning at Stormwater Treatment Area-1 West Jing-Yea Yang¹, Jie Zeng², Kwaku Oben-Nyarko¹ and Emile Damisse² ¹Stanley Consultants Inc., West Palm Beach, Florida Stanley Consultants INC. ²South Florida Water Management District, West Palm Beach, Florida Flow Rating Objective Introduction This South Florida Water Management District instigated project focuses on providing the framework for identifying appropriate streamgauging opportunities at inflow and outflow structures in STA-1 West and introduces new flow rating techniques applied to weir-box culverts at the STA. The result of such an exercise will provide for STA-1 West: (1) An The main purpose of STAs in South Florida is to remove pollutants such as $\frac{H}{h} > 1.7$ and $\frac{h}{h} \ge 1.7$ phosphorus and nitrogen from agricultural water before discharging them into the established region or range of need of flow measurements based on the operation of the STA; (2) Recommendations on future streamgauging measurements based on the rating everglades developed and used for the flow type(s) in question; and (3) Recommendations on future streamgauging based on the historical data and operational needs of the structure. Accurate flow measurements (QMEAS) in and out of these STAs, is one of the most $O = C \perp G$ $2 \pi (H - h)$ important tasks performed by the District. **Operation & Main Structures** 1 While it will seem to be a relatively simple task to "go streamgauging" at the STA structures whenever the opportunity presents itself, the actual need for The structures at G300, G301, G302 and G303 are gated nev at G301 S streamgauging at STA structure, is dependent upon the operation of the STA. Figure 9a: Controlled Submerged Figure 9b: Uncontrolled Submerged spillways which control flow in and out of STA 1West. G302 discharges the most flow into the STA. start Performing streamgauging without taking into consideration the operational needs of ell 5B the STA may lead to unnecessary repetitious measurements and an inadequacy of Table 2: Design Operations at Main Structures Enter dis starge coefficients flow predictions in terms of the range of operational requirements for the water Spillway Design Flow Rate (cfs) Headwater Tailwater 14.0 15.8 control structures tional Wildlif 15.7 14.46 Having a framework and a methodology for (1) capturing the streamgauging needs ency at G302 S v at G303 1 either from the operational plan or from historical operations and using these to (2) develop an analytical framework for identifying appropriate streamgauging $\frac{h}{d} < \frac{2}{d}$ Area (ao 1,490 941 1,876 358 562 2,293 opportunities is necessary for effectively monitoring flow through STAs. a-0.7 $\frac{h}{2} \ge \frac{2}{2} = x = att(x)$ End $Q = L \sqrt{(er^2)}$ Figure 5: Patterns at STA-1 West Spillways Based Figure 3: Schematic of STA-1 West Flows on FLOW Program Figure 10: Flow Chart and Equations of Structural Element Method for Weir-Box Culverts During the design flow condition of 3,250 cfs, CF: Controlled Free; CS: Controlled Submerged; UF: The iteration process starts with guessing the water stage average between tailwater and approximately 930 cfs (29%) will flow through the eastern flow-way, 850 cfs (26%) will flow through the western Uncontrolled Free: US: Uncontrolled Sub headwater elevation inside the weir box. The traditional weir and culvert flow ratings equations are used for each structure component. Based on the difference of flow flow-way, and 1470 cfs (45%) will flow through the discharges between these structures, the intermediate water stage is systematically northern flow-way. Inflow is distributed to the eastern and adjusted in each of the iteration step. When the discharge convergence criteria is finally western flow-ways through structure G-303, and into the satisfied, the flow rate for the weir-box culverts can be obtained. northern flow-way through structures G-304A-J. Flows passing through G-303 are distributed either south into Figure 4: Upstream View of G303 the eastern flow-way or west through structure G-255 into the western flow-way. G304 and G306 are weir-box culverts which control flow Table 1: Inflow and Outflow Structures with OMEAS distribution in the STA. These structures have weir type inlets that require a flow computation procedure different from Possible Flow Types those employed for simple structures. The inflow into the culverts is affected by the presence of the weir box at the Table 3: Design Operations at Main Structures Figure 6: Upstream View of G304A Figure 1: Arial View of STA-1West Location QA of Flow Measurements Figure 11: Comparison between Fitting Qcalc and Qcomp with respect to Omeas at STA-1West Weir-Box Culverts STA-1 West, together with its associated canals and the STA-1 Inflow Basin, is a The data validation process for the 162 measurements collected at STA-1 West involved a variety of processes, including a verification of stage(s) and gate opening(s) in DCVP as primary component of the Everglades Construction Project mandated by the 1994 Everglades Forever Act (Section 373.4592, Florida Statutes). STA-1W is located within well as measurement specific conditions in QMEAS. The historic data in DCVP provides a reference for checking the measurements' stages and gate-openings, while other Conclusion and Recommendations measurement characteristics were found in the QMEAS database Sections 1, 2, 3, 6, 7, 10, 11, 12, 13, 14, 15, 22, 23, 26, 27, 34, Township 44 South Out of 162 measurements in STA-1 West, 147 were considered good. This is 90.74% Range 39 East within Palm Beach County and is positioned immediately west of the efficiency, 3.09% of the measurements were considered unsatisfactory based on the data Arthur R. Marshall Loxahatchee National Wildlife Refuge, also known as Water validation process while 6.17% of the measurements were considered questionable. Conservation Area 1 (WCA-1). (Operation Plan 2004). Improvements are recommended for both the FLOW program and the iteration method to (HW,TW,Go, etc.) provide better results when computing the flow at weir-box culverts G304 and G306. Table 5: Number of Measurements Needed per Range of Operation per Structure Lake Okeechob Recommendations 501 8 Group Data According to Structures Flow Range Go # of Name Flow Type HWF TWE (a) (b) (cfs) Measureme Figure 8: Qmeas vs. Qcalc Graph showing Outliers (a), Revised Qmeas vs. Qcalc Graph (b) G301 Go>3.2 USF 1000-2000 17.4<Hwe<18.0 15.4<Twe<15. 5 G302 Table 4: Spillways CSF Identified Outliers and Recommendations 25-65 $12.0 \le Hwe \le 13.00$ Twe>11.0 .4<Go<6 G304A-J Free STA-1W .5<Go<6 % ERRO Type Comment 100-120 13.0<Hwa<13.85 Twee 11.0 Everglades G304A-J Submerger 50-230 12.0≤Hwe≤15.0 <Go<2.3 4 Agricultural WCA 1 G306A-J Submerged 60-105 10.5≤Hwe≤11.2 Twe>9.2 1<60<4 Area Recommendation References . Ansar M. and S. Nair. 2003. Flow computations at STA-1 west gated spillways. SFWMD 626.35 20 549

Figure 7: Flow Chart Describing Data Validation Methodology

WCA 2A

WCA 3A

Figure 2: Location of STA-1W in South Florida

Structures at which the rating equation presented larger deviations with respect to the measurements (R²<0.95), were further investigated by recalibrating rating currently used in the flow program or introducing a new algorithm to suit the particular structure site. Outliers to such a recalibration were again investigated and recommendations made, in comparison to similar measurements at the same culvert site.

technical publication

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2. Damisse and Fru, 2006. Improved Flow Computation at District Culverts. Technical

2004, Operation Plan, Stormwater Treatment Area 1 West, South Florida Water

Zeng, Jie, 2007, Report of Implementation of Iteration Algorithm for Weir-Box Culverts.